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# International Review of Civil Engineering (IRECE)

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# Effect of Aluminum Waste on Mortar Thermo-Mechanical Behavior

Mekki Maza<sup>1</sup>, Nadia Tebbal<sup>2</sup>, Zine El Abidine Rahmouni<sup>1</sup>

Abstract – This study examines the effect of aluminum (Al) waste additions on the mechanical performance of mortars at high temperatures. The tested mortars have been formulated with different proportions (0%, 2.5%, 5%, 7.5%, and 10%) by weight of sand after being exposed to five temperatures (50 °C, 150 °C, 200 °C, 400 °C, and 600 °C) without imposed load during heating. Workability, setting time of cement, air content, density, mass loss of mortar, thermal conductivity, porosity and mechanicals strength have been examined. The test results indicate a considerable decrease in workability and strength density of the mortar with the addition of Al. This composite has a well thermal conductivity result with 2.5Al and environmentally friendly than ordinary mortar. Further, the experimental data obtained have suggested that the compressive and the flexural tensile strength have been significantly reduced by 90 % in the mortar samples incorporating 10% Al after being exposed to the high temperature of 600 °C. Moreover, the mechanical strength of that mortar has been quite high at the age of 28 days at elevated temperatures in comparison with that measured at 20 °C. The strength of the mortar with Al can be sufficient for some applications where a lightweight, low-strength mortar is required. The use of Al in the production of low-strength concrete can contribute to more sustainable construction. Copyright © 2023 Praise Worthy Prize S.r.l. - All rights reserved.

Keywords: Mechanical Strength, Aluminum Waste, High Temperatures, Porosity, Mortar

# Nomenclature

Al	Aluminum
CPJ CEM II 42.5	Portland cement of strength class
	42.5
SP	Superplasticizer
DS	Dune Sand
CS	Crushed Sand
$M_f$	Fineness modulus
СМ	Control Mortar
MA2.5	Mortar with 2.5% Aluminum by
	weight of sand
MA5	Mortar with 5% Aluminum by
	weight of sand
MA7.5	Mortar with 7.5% Aluminum by
	weight of sand
MA10	Mortar with 10% Aluminum by
	weight of sand
W/C	Water/Cement ratio
Standard ASTM C	Standard American Society for
	Testing and Materials Cement
NF EN	French Standard being the
	translation of a European Standard
k	Thermal conductivity coefficient
Q	Heat conducted through
	the samples
L	Specimen thickness
Α	Cross-section area
$\Delta T$	Difference in temperature on both
	sides of the sample

C <sub>3</sub> A	Tricalcium aluminate
$C_2S$	Dicalcium silicate
C <sub>3</sub> S	Tricalcium silicate
CSH	Hydrated Calcium Silicate
RH	Relative Humidity
Р	Porosity
Wt	Weight
Ŷ	Absolute density [g/cm <sup>3</sup> ]
0	Bulk gravity [g/cm <sup>3</sup> ]
ASTM E 119	Standard Test Methods for Fire
	Tests of Building Construction and
	Materials

# I. Introduction

Aluminum recycling is not a new practice; it dates back to the early 1900s and was widely used during World War II. However, it was a low-profile activity until public awareness was raised [1]. The use of aluminum alloys in engineering applications, primarily in residential building components such as facades, siding, windows, doors, and railings, results in tangible waste that poses an environmental problem and requires management and recycling [2]. Many studies have used aluminum dross in construction applications to replace sand or cement, produce concrete blocks, make aluminate cement, or as filler in asphalt products. Puertas et al. have proved that aluminum dross comprising high alumina could be used as a raw material in the cement production industry [3]. Alzubaidi has investigated the research of recycling aluminum by-product waste in concrete production (ALBP). The test results indicate a decrease in workability, compressive strength, flexural strength, and density of plain concrete with the addition of ALBP [4]. Borhan et al. have studied the thermal properties of cement mortar containing aluminum fine aggregate waste. The results have also revealed that replacing sand with 40 and 60% aluminum filler decreases the reduction rate in compressive strength when the tested samples immediately after being heated to  $55^{\circ}$ C in an oven [5]. Regarding the thermal conductivity test, the results show a decrease with the increase in aluminum waste content [5]. The effect of temperature on the strength property of concrete mixed with aluminum can waste was conducted by Son in 2004.

The results indicated that the strength of the molds heated to 300 °C decreased by 10% compared to the normal mold, 35% for 600 °C, and 85% for 900 °C. The results also indicated that molds containing more than 5% aluminum could not be formed, their strength was much lower than normal molds, and they deformed greatly with higher centigrade temperatures [6]. The results have showed that AW could be used as a retarder and, therefore, a good material for concreting in hot weather [7]. Concrete can deteriorate for various reasons, and concrete damage often results from a combination of factors. Very little research has been conducted to investigate the influence of various material parameters on the durability performance of concrete or mortar mixtures, including the effects of Aluminum Fines (AF) on freeze-thaw resistance. The freeze resistance of concrete made with durable aggregates is determined by the ability of the air-vacuum system to prevent the development of destructive pressures due to freezing and the associated moisture movement into the concrete pores [8]. The use of a superplasticizer admixture in concrete has resulted in a significant increase in slump and flow of the mix but has reduced the air content of the mix and increased its density. With tilting the airentraining admixture, the mix's slump, flow, and air content have increased, but the density has decreased [9].

According to [10] aluminum industry flowchart, the hydraulic activity of aluminum drosses allows it to be used as a cement hardening agent without clinker, with a suitable activator and mix design providing quite usable mechanical properties and high resistance to water and sulfate attack. However, the importance of aluminum residues has not been limited to improving certain properties of concrete but has also surpassed it to increasing the bond between lateralized concrete and steel [11]. The use of nanomaterial has become a popular way to improve the performance of cement-based composites. At the same time, ultra-high-strength concreteis increasingly being used. [12] has studied ultrahigh strength cement-based composites designed with aluminum oxide nano-fibers. This research has demonstrated that using small amounts of aluminum oxide nanofibers in an oil-well cement-based mortar could provide compressive strength close to 200 MPa and help meet the benchmark for ultra-high-strength

cement-based composites. [13], [14] have conducted a study on concrete produced using recycled aluminum dross for hot weather concreting conditions using mixing and testing as recommended procedures by relevant codes. The results have indicated that the initial setting time of recycled aluminum dross concrete has been extended by approximately 30 min at the 20% replacement level. This recycled aluminum foam concrete property is suitable for hot weather concreting conditions. Aadi et al. [15] have studied the use the natural sand replaced by Aluminum Wastes (AW) to produce green cement mortar for different construction applications. Their results have showed that the mortar contained up to 5% of AW can be used for structural application. The AW ratio in the range of 15-30% can be suitable for lightweight mortar for nonstructural applications. According to a number of researchers, the categorization of concrete is often based on its specific gravity and compressive strength [16]-[18].

Mohammadyan-Yasouj [19] has showed that the using Nano-Alumina (NA) decreases compressive strength of samples in the ambient temperature, but improves their compressive strength after ten minutes heating under the target temperature of up to 200 °C. Studies on the effect of aluminum waste on the thermo-mechanical properties of mortar are scarce. To this end, this study aims to evaluate the behavior of fresh and hardened mortar manufactured by adding industrial by-product (Al) waste in different proportions. Two critical aspects of mortar durability performance have been investigated: formulated mortar rheology and fire resistance. The experimental parameters have included the cement type and fine aluminum amount. Different types of mortar M have been made from materials and products manufactured in Algeria: Portland Artificial CPJ CEM II 42.5 cement, Superplasticizer (SP), Aluminum waste (Al) with binary sand mixtures. In this study, binary sand mixture aggregates have been used: DS (Dune Sand) and CS (Crushed Sand). The goals of this experimental work have been to investigate the following aspects:

- The possibility of reusing aluminum waste in mortar and concrete mixes as a partial replacement of fine aggregates in order to reduce the environment impact resulting from industrial waste disposal;
- The impact of solid wastes (aluminum waste) on the physical properties of binary sand prepared (i.e., density, porosity, fineness modulus, particle size distribution and water absorption) and mechanical response (i.e., setting time and air content, flexural, tensile and compressive strengths, Influence of temperature and heating time and mass loss) of the mortars made with binary sand mixtures.

In order to protect the environment from industrial waste and to explain the importance of reusing aluminum waste in mortar in particular and innovative concrete in general, the paper has been divided into different sections. The first part deals with all recent studies regarding the valuation of industrial residues in the field of cement materials, especially aluminum residues. The second section, "Materials and Methods," deals with the materials used, their chemical and physical properties, and illustrates methodology of mixing all mixtures.

Visual evaluations of all formed mortars before and after hardening, as well as exposure to high temperatures are discussed in the following section. Discussion and conclusions are summarized at the end of the paper.

# **II.** Materials and Methods

# II.1. Materials

*Portland cement:* in this experimental study, the type CEMII 42.5 from Hammam Dalâa local factory has been used. It has a Blaine-specific surface area of  $3,850 \text{ cm}^2/\text{g}$  and a density of 3.2. Table I lists the chemical compositions of the cement.

Sand: a mixture of 1/3 dune and 2/3 crushed sand [20] has been used for the preparation of the various study mortars with fineness modulus ( $M_f$  of 2.63). The natural fine aggregates (DS) used have been dune sand with a particle size between 0.08 mm and 5 mm. This natural sand has been collected in the region of Boussâada (250 km east of Algiers). The particle size analysis has been performed according to the European standard (NF EN 933-1) [20].

*Crushed Fine Aggregate (CS)*: in this study, the fine aggregate manufactured used have been crushed sand generated from the quarry waste (calcareous crushed sand). Figs. 1 and 2 show the particle size distribution of the sand used. Table II presents the physical properties of dune sand, crushed sand, and aluminum waste.

TABLE I							
THE CHEMICAL AND MINERALOGICAL							
PROPERTIES OF CEMENT							
		Chemi	cal comp	osition (	%)		
SiO <sub>2</sub> CaO Al <sub>2</sub> O <sub>3</sub> Fe <sub>2</sub> O <sub>3</sub> MgO SO <sub>3</sub> Na <sub>2</sub> O K <sub>2</sub> O						0	
17.8	61.2	4.4	2.6	1.8	2.6	0.1 0.0	б
	I	Mineralo	gical con	position	s (%)		
$C_3S$	C <sub>2</sub> S C <sub>3</sub> A C <sub>4</sub> AF		Gypse				
45.8	14.	3	2.6	10		19.8	
TABLE II Physical Properties Of Dune Sand, Crushed Sand, And Aluminum Waste							
Pł	nysical pi	operties		DS	CS	Al	
Fineness modulus		1.77	3.16	5.09			
Absolute density (g/cm <sup>3</sup> )		n <sup>3</sup> )	2.65	2.70	) 5.45		
Apparent density (g/cm <sup>3</sup> )		n <sup>3</sup> )	1.52	1.49	2.07		
	Porosit	y (%)		42	44	62.01	
	SC				2		



Fig. 1. Particle size distribution of sand used



Fig. 2. Particle size distribution of Al waste

Aluminum waste has been obtained from the remains of aluminum windows and doors manufacturing aluminum. The substitution ratios of aluminum waste in mortar have been 0, 2.5, 5, 7.5, and 10% by volume of sand. It has been screened on the sieve number (0/3 mm) to be close to the sizes of sand (Fig. 2). Table III summarizes the chemical composition of Al used. Tap water from the civil engineering laboratory has been used for mixing throughout the study.

## II.2. Mix Design

Table IV lists the proportions of the mortar mixes.

Five mortar mixes have been prepared for the experimental stage. The Control Mix (CM) has included only Portland cement as a binder, i.e., it has not contained aluminum. The mixes had different aluminum contents of 2.5, 5, 7.5, and 10% by weight of sand and have been named MA2.5, MA5, MA7.5, and MA10, respectively. The total amounts of cementitious material content and slump value have been recorded consistently for all mixes. Since the slump value has been kept constant, water has been gradually added to the mixes, and thus the water/cement ratio (W/C) has not been kept constant and has changed from 0.5 to 0.6 (Standard ASTM C 305-99) [21]. The fresh mortar mixtures have been prepared in several steps.

TABLE III						
Снем	CHEMICAL COMPOSITION OF ALUMINUM WASTE				STE	
]	Elements			(9	%)	
	Si			.5	16	
	Fe			.335		
	Cu			.0771		
	Mn			.094		
	Mg			.3	27	
	Zn			.9	58	
	Ti			.0	197	
	Cr .0013			013		
	Ni .0043			043		
	Pb .0063			063		
	Sn .007			07		
	Na .0068			068		
Al 98.51			.51			
TABLE IV						
Mix Design	Sand (kg)	AL (kg)	Water (kg)	W/C	Cement (kg)	
MC	1.350	0	225	0.50	0.000	
MA2	1.316	0.034	270			
MA5	1.283	0.06750	270	0.60	0.450	
MA7.5	1.249	0.10125	270			
MA10	1.215	0.135	270			

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The sand and cement to be tested have been mixed with water in the following proportions:  $450 \pm 2$  g of cement and  $1350 \pm 5$  g of sand. The mixing procedure has been determined by verifying the evolution of the slump flow as a function of mixing time. Initially, the dry materials have been mixed for 15 s.

Then, water has been added at a mixing speed of 1.6 m/s, and the superplasticizer has been manually added to the mixture within 20 s.

Setting time and air content: the testing of the initial and final experimental protocol setting has been consistent with the recommendations of the European Standards NF EN - (196- 3) and NF EN (197- 1) [22], [23]. The mortar air content has been determined following Standard ASTM C 185.

*Density*: the density  $\rho$  of the mortars has been measured according to the Standard NF EN 196-1 [24].

Compressive and bending tensile strength: the mechanical resistance has been tested according to Standard NF-EN 196-1 [24]. For 24 h before demolding the mortar specimens,  $4 \text{ cm} \times 4 \text{ cm} \times 16 \text{ cm}$ , have been kept in a wet place at a temperature of 20 °C and 95% Relative Humidity (RH) during 24 hours. Then they have been kept under water until the age of testing. After 28 days, they have been tested in a three-point bending machine.

*Porosity*: the water-accessible porosity protocol is in accordance with the AFREM group recommendations [25]. Porosity has been calculated from the absolute and bulk density values by using Eq. (1):

$$P(\%) = \left(1 - \frac{\rho}{\gamma}\right) 100 \tag{1}$$

where *P* is the porosity that corresponds to the pore and void content of the samples (wt. %),  $\gamma$  is the absolute density (g/cm<sup>3</sup>), and  $\rho$  is the bulk gravity (g/cm<sup>3</sup>).

*Heating:* after 28 days, specimens with dimensions  $4 \text{ cm} \times 4 \text{ cm} \times 16 \text{ cm}$  have been dried in an oven at 100 °C until stabilization of their mass. All specimens have been subjected to high temperatures (200 °C, 400 °C, 600 °C, and 900 °C) according to the Standard ASTM E 119-00 [26].

*Thermal conductivity*: the thermal conductivity coefficient has been tested by using the steady-state guarded hot plate method ASTM C 177-19 [27]. The thermal conductivity (k) has been calculated by using Eq. (2):

$$K = QL/A\Delta T \tag{2}$$

where Q is the amount of heat conducted through the samples, L is the specimen thickness, A is the cross-section area, and  $\Delta T$  is the difference in temperature on both sides of the sample.

## **III.** Results and Discussion

# III.1. Workability of Fresh Concrete

Figure 3 presents the slump results for all mortar mixtures. The slump value for the control mortar has

been 6 cm. while those for MA5 to MA10 have been observed at 5.5, 5.3, 5, and 4.8 cm. respectively. The addition of Al decreases the mortar slumps compared to the control, which can be attributed to two reasons. The mechanical reason concerns the varying sizes of the aluminum particles that consume part of the volume of the paste to coat it and mechanically hinder the mortar flow. The chemical reason is the reaction between metallic aluminum and water in the alkaline medium, forming aluminum hydroxide and releasing hydrogen gas. The aluminum hydroxide passes into the solution and disturbs the alumina-sulfate balance ratio in the mortar, disturbing the setting properties and forming compounds, reducing slump and potentially important for rapid setting as seen at the high aluminum dosage of 10%. The addition of aluminum waste in the 5% to 7.5% range results in similar reductions in a slump. Beyond this range, aluminum waste significantly reduces the mortar slump, and the mortar no longer becomes workable [28].

# III.2. Setting Time of Cement

Figure 4 shows the setting time properties for all mixes. It can be seen that when the aluminum waste reaches 10% of the sand replacement, the Al content increases hardening and reduces setting time. A higher surface area of nanoparticles in the aluminum waste during the hydration process can be explained as a major reason for the extension or shortening of the setting time, allowing for greater absorption of liquids [29].

#### III.3. Air Content

Figures 5 show the effect of Al on-air content and mortar swelling for the five mortar mixes containing 0, 2.5, 5, 7.5, and 10% Al. The measurements have been taken after casting. The swelling test has been performed after the mortar sample dimensions have been stabilized.

From the results obtained, it can be seen that an increasing amount of aluminum waste reduces the setting time of the mortar.



Fig. 3. Effect of Al % on concrete slump values

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Fig. 4. Effect of Al % on mortar setting time values

The high porosity or trapped air content can be explained by the high absorption capacity of aluminum waste (Fig. 5(a)). The expansion of the mortar samples for the five mortar mixtures containing 0, 2.5, 5. 7.5 and 10% Al has been measured after casting (Fig. 5(b)). The expansion test has been performed after the dimensions of the prismatic and cylindrical mortar specimens had stabilized.

The growth of the specimen height has been measured 30 min after pouring. For all mortars, the volume increases with increasing Al % in the mixtures.

The maximum expansion value of all mixtures has been 6 cm for MA10. The expansion is attributed to hydrolysis of Al and Al<sub>2</sub>O<sub>3</sub> since the medium is alkaline to form the voluminous Al(OH)<sub>3</sub>, in which the hydrogen gas creates bubbles that try to escape to the surface, thus, creating voids and increasing the volume of mortar.

Cesar (2002) explained this expansion reaction between alumina and cement hydrates that can form early ettringite that is expansive, thus increasing the volume of mortar, similar to the case when expansive cement is used in a mortar [30].

# III.4. Mortar Density

Figure 6 shows the density values of the hardened mortar. It can be noted that the mortar density decreases with the addition of Al. The maximum reduction has been found at around 27% when Al has been added at 10% of the sand weight. Most researches have attributed the decrease in density to the presence of a reaction between the metallic aluminum and the alkaline solution of the cement paste. The increase in the volume corresponding to a reduction in mortar density is due to the release of hydrogen gas and the formation of expansive products from the exothermic reaction between aluminum and cement hydrates [31] according to the following endothermic reaction:

$$2AI + 3Ca (OH)_2 \rightarrow 3(CaO)AI_2O_3 + 3H_2 \qquad (3)$$



(b) Samples after expansion

Figs. 5. (a) Effect of Al% on mortar air content and (b) volume height values



Fig. 6. Effect of Al % on mortar density

#### III.5. Effect of Aluminum Waste on the Porosity

Figure 7 shows the results of the porosity tests. The percentages of porosity in MC, MA2.5, MA5, MA7.5 and MA10 have been 8, 9, 11.75, 13.50, and 16.44%, respectively. In the short term (28 days), the MC mortar has low porosity than the MA mortar. The difference between the physical properties of each type of fine aggregate is the cause of the variance observed between the porosities studied. In addition, it can be noticed that partial replacement of sand with aluminum aggregate (2.5, 5, 7.5 and 10% Al combined) increases porosity at 28 days. However, at 10% aluminum waste, there is a significant increase in porosity (for MA10 mortar, the increase in porosity is 50%) due to the fine aluminum aggregate's high absorption capacity and high porosity.

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Fig. 7. Effect of Al % on mortar porosity

# III.6. Effect of Aluminum Waste on the Mechanical Strengths

Figure 8 shows the mortar compressive and flexural strength results at 28 days. It can be noted that the compressive strength of the mixtures decreases with the addition of Al, and the maximum reduction has been recorded when Al has been added at 10% of the sand weight.

The strength of MA2.5 has been up to 16 MPa, while the resistance of MA10 has been only 4.55 MPa, which is proved by the rapid crushing of the samples (Fig. 9).

Therefore, it can be concluded that replacing sand with high levels of aluminum waste is not suitable (Watanabe et al. 2006). Figure 10 shows the aluminumwater reaction. Hydrogen gas and aluminum hydroxide have been produced in the chemical reaction between water and metallic aluminum. The release of hydrated alumina and aluminum hydroxide into the solution disrupts the equilibrium between the tricalcium aluminate  $(C_3A)$  and sulfate (gypsum) phase [32]. This leads to the formation of delayed ettringite, which disturbs the microstructure, and reduces the concentration of calcium oxide partially consumed to enhance the reaction with aluminum. Thus, the decrease in resistance is explained by the reduction in the amount of calcium silicate hydrates ( $C_2S$  and  $C_3S$ ), which is mainly responsible for the strength of the mortar [33]. The flexural strength of the control mix has been approximately 5.65 MPa at 28 days of age. At this rate of addition, the flexural strength of the mortar at 28 days has been 1.12 MPa, which corresponds to an 80% decrease in strength compared to the control. Coupled with the compressive strength results, the flexural strength has decreased with adding Al, and the trend is similar to the compressive strength.

Recent research suggests some uses where residual flexural strength of about 2.0 MPa may be of interest for non-load-bearing walls and to produce non-load-bearing concrete blocks, which can be achieved with Al at about 10% [32], [33].



Fig. 8. Effect of aluminum waste on the mechanical strengths



Fig. 9. Mortar samples broken during mechanical testing



Fig. 10. Aluminum-water reaction

## III.7. Influence of Temperature and Heating Time on Mortar Mass Loss

Figure 11 shows the effect of high temperatures on the mass loss of MC, MA2.5, MA5, MA7.5, and MA10. The weights of the specimens have been evaluated in four distinct temperature ranges: 100, 150, 200, 400, and 600 °C. The other three samples have been placed in an oven with a temperature up to 55 °C (the duration of the peak period in Ouargla. Algeria).

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Fig. 11. Evolution of mass loss related to temperature

The mass loss has increased for air-cooled samples with increasing temperature for all samples subjected to high temperature. In the range of 50-150 °C, MA10 mortar shows the largest decrease. At the same time, the control mortar and MA2.5 have showed the minimum value. MA7.5 and MA5 mortars have showed higher weight loss than MA2.5 and lower than MA10. The weight loss in this temperature range is generally attributed to moisture evaporation from the sample surface to the atmosphere. When the temperature rises from 150 to 400 °C, most of the water in each concrete specimen evaporates.

The increase is almost linear up to a temperature of 600 °C. This result can be explained by the evaporation of water and the gradual dehydration of the CSH gel [34], [35]. Figure 12 shows that increasing aluminum increases void formation due to gas release, reducing the mortar mass at high temperatures. The mass loss is minimal for mortars (MA2.5) up to about 600 °C.

However, the aggregate's nature significantly influences the mass loss in mortars above 600 °C. In the case of aluminum scrap mortar, the mass loss is not significant above 600 °C compared to MC mortar. Many researchers have confirmed that the higher percentage of mass loss in concrete/mortar is attributed to the dissociation of dolomite in the carbonate aggregate at around 600 °C.

The increase in porosity with increasing temperature and pore evolution is contaminated due to product deterioration in the maximum degrees of temperature [36].

# III.8. Results of Compressive Strength After Cooling

Figure 13 shows the compressive strength of mortars at  $20 \pm 5$  °C and after exposure to 200, 400, and 600 °C.

It can be noted that the strength decreases with increasing temperatures.



Fig. 12. Porous texture of mortars



Fig. 13. Compressive strength after cooling

Mortars exposed to the influence of high temperatures lose a significant portion of their strength value. The loss of compressive strength becomes evident at 150 °C with a strong recovery of 12.5, 17, and 38.5% for MC, MA2.5, and MA5 mortars, respectively. These gains at 150 °C to 200 °C are attributed to the increase in forces between the gel particles due to the removal of water content [37]. When the temperature increases above 150 °C, the strength of the mortars is greatly reduced due to the dehydration of the surface water content. Between 400 and 600 °C, the free water content evaporates quickly and causes internal cracking due to the steam effect, which explains the decrease in resistance. Severe strength losses have occurred in all MC, MA2.5, MA5, and MA7.5 mortars. In this range, the cement paste contracts while the aggregates expand. Thus, the transition zone and the bond between the aggregates and the paste are weakened. MA10 mortar has showed the greatest loss of strength, approximately 80%, 86%, and 100% of its unheated strength when heated to 200, 400, and 600 °C, respectively. Ca(OH)<sub>2</sub> decomposes between 400 and 600 °C [38], [39]. For 400 °C, a strong

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compressive loss for MA10 specimens has been tested.

All mortars (MC, MA2.5, MA5, MA7.5) have lost 100 % of their resistance at 600 °C. It can be noted that the great losses of resistance for all mortars are due to the porous microstructure of mortars, which has caused the accumulation of high internal pressure attributed to the transition of water vapor from the intercalated water.

Thus, this process causes severe deterioration and strength losses in the mortar after exposure to high temperatures. It appears that the amount of aluminum waste does not significantly influence the compressive strength between 400 °C and 600 °C. However, in the range of 50 °C and 200 °C, 2.5% to 5% Al significantly affects the compressive strength.

#### III.9. Thermal Conductivity

Figure 14 shows the thermal conductivity results for MC, MA2.5, MA5, MA7.5, and MA10 mortars with partial sand replacement. For mortars without Al, the thermal conductivity has been  $1.93 \pm 2$  W/mK. However, the thermal conductivity of mortars with Al has been generally in the lower range of about 1.9 -1.85 W/mK.

The thermal conductivity values obtained are in good agreement with those previously reported by Borhan et al. [40], [41]. The decrease in thermal conductivity has been approximately 15.54 to 41.45%. This minor conductivity of the MA mixed mortar indicates that the presence of aluminum content has a positive effect on decreasing the capacity of the heat transmitted during the thickness of the samples. The low results of the thermal conductivity coefficient in the mortar can be explained by an increase in the formation of voids due to gas release.

As a result, this mortar has a better insulating competence than the normal mortar. According to Figure 14, the substitution of sand for aluminum waste of more than 10% generally reduced the thermal conductivity.

The only exception has been the case of MA2.5 mortar in the mortar containing 2.5% Al, which may be due to the filling of voids in the binder matrix with the small amount of aluminum waste, resulting in high thermal conductivity.



Fig. 14. Thermal conductivity of mortars without Al and with 2.5, 5. 7.5, and 10% Al

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For the other mortars, the reduction in thermal conductivity with the addition of Al is due to the introduction of voids in the mortars, resulting in increased porosity and decreased compressive strength of the mortars [42], [43].

# **IV.** Conclusion

This study has investigated the mechanical and thermal behavior of a new type of mortar obtained by adding aluminum waste. In addition, the effect of the severe environmental conditions of the Algerian desert on the durability properties of the added aluminum waste mortar has been studied. The results obtained encourage the mixing of aluminum residues in the mortar, which greatly contributed to its workability. In addition, the cement paste's initial and final setting time increases with the incorporation of the aluminum waste mixture, and up 10% of the weight of the sand, the compressive and flexural strengths reduces to acceptable levels for certain applications (building subflooring, block, and precast panel manufacturing). Furthermore, the higher percentage of Al waste replacement is, the more air is trapped and the higher the porosity is, which negatively affects the mechanical strength. In the context of thermal conductivity, the results of laboratory experiments have demonstrated that replacing part of the sand in a mortar of less than 10% had a positive effect on increasing thermal conductivity. With these encouraging values, the material developed in this research using local materials such as sand dunes, very abundant in the Sahara of Algeria, shows that it is possible to consider this type of concrete for the production of lightweight concrete that adapts very well to the hot and arid environment of our region given its very acceptable characteristics, which allow its classification in the margin of lightweight concretes. In future studies, it is recommended adding Al waste to environmentally friendly geopolymer activation solutions in order to reduce the geopolymer preparation time and induce significant changes in porosity and it is also possible to recommend the production of cellular concrete.

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